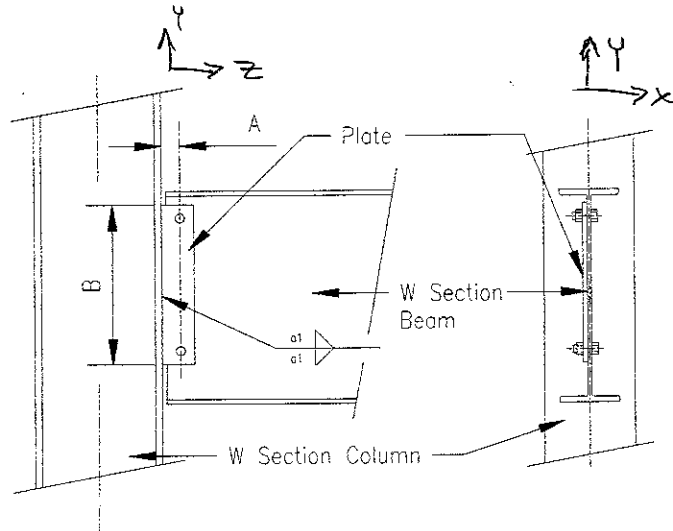


Example Problem 5.5

Given: A connection plate is welded to the face of a W section column flange and is bolted to the web of a W section beam as shown. The weld electrode is F7. $A = 2$ inches and B is to 9 inches. The weld size, a_1 , is $1/4$ inch. The beam reaction transferred by the connection plate is 3 parts dead load and 5 parts live load.



Wanted:

- a) Determine the maximum beam reaction based on weld strength of the fillet welds that connect the plate to the W section. Use LRFD and express your result in terms of comparable service level loads.
- b) Repeat the problem using the coefficients on SCM pg 8-66.

Solution:

DETERMINE THE COMPOSITE LOAD FACTORS:

LC2 CONTROLS FOR BOTH LRFD & ASD.

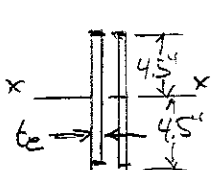
LRFD: $P_u = 1.2(3/8) + 1.6(5/8) = 1.45 P_{s,eq}$

ASD: $P_a = 1(3/8) + 1(5/8) = 1.00 P_{s,eq}$

PART a) FIND MAX STRESS WHEN P_u / K THE SCALE TO FIND CAPACITY:

$R = 1 K ; M = (1K)(2") = 2"K$

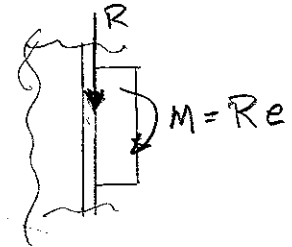
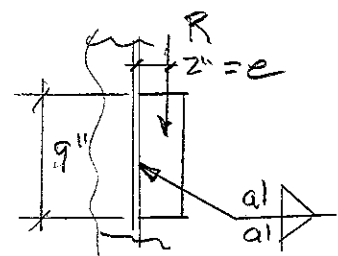
WELD SECTION PROPERTIES:



$t_e = .707(1/4") = .177 in$

$A_w = 2 t_e l = 2(.177)(9") = 3.18 in^2$

$I_x = 2 \frac{t_e l^3}{12} = \frac{6(.177)(9")^3}{6} = 21.5 in^4$



TRANSLATION STRESS:

$$f_y = \frac{R}{A_w} = \frac{1k}{3.181in^2} = .314 ksi$$

DIRECTION IS IN +Y DIRECTION

MAX BENDING STRESS

$$f_z = \frac{Mc}{I} = \frac{(2"K)(4.5")}{21.5in^4} = .524 ksi$$

DIRECTION IS IN +Z DIRECTION

VECTOR SUM:

$$f_w = \sqrt{(.314)^2 + (.524)^2} = .524 ksi$$

WELD CAPACITY

$$F_w = .6 F_{EXX} = .6(70 ksi) = 42 ksi$$

$$\frac{R_N}{F_w} = \frac{1k}{f_w}$$

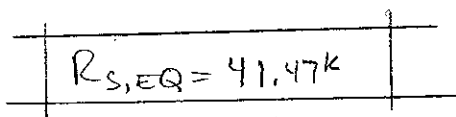
$$R_N = \left(\frac{42 ksi}{.524 ksi} \right) (1k) = \underline{80.17k}$$

LRFD

$$\phi R_w = .75(80.17k) = 60.13k$$

$$CLF = 1.45$$

$$R_{S, EQ} = \frac{60.13}{1.45} = 41.47k$$

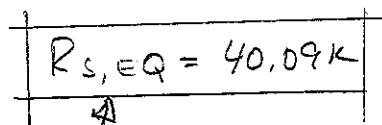


ASD

$$R_w / \Omega = \frac{80.17k}{2.0} = 40.09k$$

$$CLF = 1.0$$

$$R_{S, EQ} = \frac{40.09}{1.0} = 40.09k$$



↑ ANSWER (a)

PROBLEM 5.5 (CONTINUED)

↓BQ 3/3

PART (b): SEE INSTRUCTIONS ON SCM pg 8-66

$$k=0; C_1=1; D=4$$

$$a = \frac{2''}{9''} = 0.222$$

FIND "C" BY INTERPOLATION

a	C
0.200	3.51
0.222	3.42 → C
0.250	3.31

$$R_N = CC_1Dk = (3.42)(1)(4)(9)$$

$$R_N = 123.2K$$

LRFD

$$\phi R_N = 0.75 (123.2K) = 92.37K$$

$$CLF = 1.45$$

$$R_{s,eq} = \frac{92.37K}{1.45}$$

$R_{s,eq} = 63.70K$

↑ ANSWER (b)

ASD

$$R_N/S_1 = \frac{123.2K}{2.0} = 61.58K$$

$$CLF = 1.0$$

$$R_{s,eq} = \frac{61.58K}{1.0}$$

$R_{s,eq} = 61.58K$

↑